Soil cohesion value table pdf file format

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N Allowable Bearing Pressure for Foundations on Non-Cohesive Soil

The general route for establishing the *allowable bearing pressure* is as follows.

(1) Divide the *ultimate bearing capacity* by a factor of safety (typically 3.0), to obtain the *safe bearing capacity*.
(2) By looking at peedicted values for settlement, determine the bearing pressure which corresponds to an acceptable level of settlement.
(3) The *allowable bearing pressure* is the *lower* of the two values obtained from (1) and (2).

In section 2.3.5 *Safe bearing capacity – cohesionless soils*, it states that:

> 'foundation design on non-cohesive soil is usually governed by acceptable settlement, and this restriction on bearing pressure is usually much lower than the ultimate bearing capacity divided by the factor of safety of 3. Generally only in the case of narrow strip foundations on loose submerged sands it is vital to determine the ultimate bearing capacity, since this may be more critical than settlement'.

This indicates that a *settlement* rather than a *bearing capacity* calculation would be the normal route for establishing the allowable bearing pressure for sands and gravels. Because of the uncertainties and assumptions involved in detailed settlement calculations, this approach is normally short-circuited by use of the Terzaghi and Peck allowable bearing pressure chart in Fig. N.1.

The allowable bearing pressures in the chart assume a maximum settlement of 25 mm, which experience has shown is a satisfactory value for maintaining total and differential settlements within acceptable limits. They also assume the water-table is at least a depth of B below foundation level; if the water table is at or close to the foundation level then the allowable bearing pressures indicated should be halved.

To use the chart, an SPT value is obtained from the soils investigation report. The proposed width of base, together with the SPT value, are used to read off an allowable bearing pressure. This is then checked to ensure it exceeds the applied bearing pressure; if not the base length and/or width is increased, and the process repeated until a satisfactory base size is obtained.

This process is illustrated in the following worked examples.

Worked Example 1: Square pad base

A pad foundation is required to support a superstructure load of $P = 1500$ kN. The soils investigation indicates 0.9 m of topsoil and silty clay overlying a considerable depth of medium dense sand. Average SPT values for the top metre of sand are in the range $N = 22 - 41$; a conservative average value of $N = 25$ will be assumed for determining the allowable bearing pressure.

A square base of 2 m × 2 m is initially assumed. From Fig. N.1, the allowable bearing pressure is $n_a = 280$ kN. This gives a capacity of

$P_a = n_a BL$
$= 280 \times 2.0 \times 2.0$
$= 1120$ kN $< P = 1500$ kN $\Rightarrow$ Not enough

At this allowable bearing pressure, the required area of a sqaare base would be

$A = P/n_a$
$= 1500/280$
$= 5.36$ m^2
$= 2.31$ m × 2.31 m

However examination of Fig. N.1 indicates that a larger width of base will result in a lower allowable bearing pressure. A 2.4 m × 2.4 m base will therefore be assumed; from Fig. N.1 this gives an allowable bearing pressure of $n_a = 270$ kN. The actual bearing pressure is

$n = P/A$
$= 1500/(2.4 \times 2.4)$
$= 260$ kN/m^2 $< n_a = 270$ kN $\Rightarrow$ OK

and the area of the base is

$A = BL$
$= 2.4 \times 2.4$
$= 5.76$ m^2

Worked Example 2: Rectangular pad base

The previous example will be reworked for a rectangular base, whose width is limited by site constraints to B = 2.0 m. A base size of 2.0 m × 2.5 m is initially assumed. From Fig. N.1, the allowable bearing pressure is $n_a = 270$ kN. This gives a capacity of

$P_a = n_a BL$
$= 270 \times 2.0 \times 2.5$
$= 1350$ kN $< P = 1500$ kN $\Rightarrow$ Not enough

Increase length of base by the ratio P/P_a

$L = 2.5 \times (1500/1350)$
$= 2.8$ m

This results in a bearing pressure of $n = n_a = 270$ kN/m^2, and a base area of

N Value	Friction Angle, φ' (Deg.)	Relative Density, D_r (%)	Description
Less than 4	25 - 28	Less than 15	Very loose
4 - 10	29 - 32	15 - 60	Loose
10 - 30	33 - 35	60 - 75	Medium
30 - 50	36 - 40	75 - 90	Dense
Over 50	41 - 45	Over 90	Very dense

x $^{-3}$)	Average $\rho_{d\,max}$ (g·cm^{-3})	Proctor modified test	
3	1.859	III	9
5		IV	1
3	1.832	B+A	8
.1		B+B	8

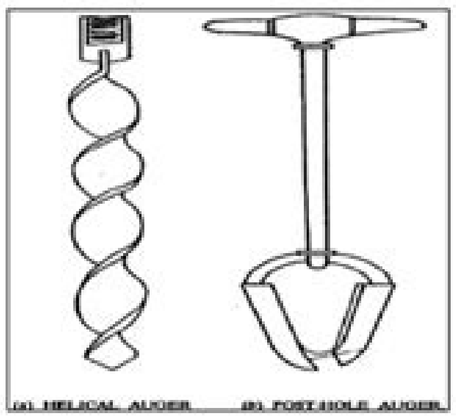
Table 2. Undrained monotonic test results for different initial relative densities of sand-silt mixtures.

	F_c (%)	e	e*	e^*_{min}	e^*_{max}	D_r(%)	D^*_r(%)	M	S_{us}/σ_c
Clean Sand (0%F_c)	0%	0.795	0.795	0.519	0.844	15%	15%	0.84	0.1615
		0.665	0.665			55%	55%		0.1811
Silty Sand (10%F_c)	10%	0.742	0.897	0.591	0.951	15%	-16.26%	0.83	0.1506
		0.610	0.753			55%	27.98%		0.1682
Silty Sand (20%F_c)	20%	0.703	0.978	0.648	1.037	15%	-41.34%	0.82	0.1347
		0.569	0.823			55%	6.56%		0.1466
Silty Sand (30%F_c)	30%	0.678	1.051	0.712	1.111	15%	-63.66%	0.80	0.1149
		0.548	0.892			55%	-14.62%		0.1303
Silty Sand (40%F_c)	40%	0.701	1.167	0.856	1.222	15%	-99.38%	0.78	0.1288
		0.586	1.021			55%	-54.37%		0.1444

A) Explain 3 types of boring for cohesive and cohesion less soil (include picture for your explanation)

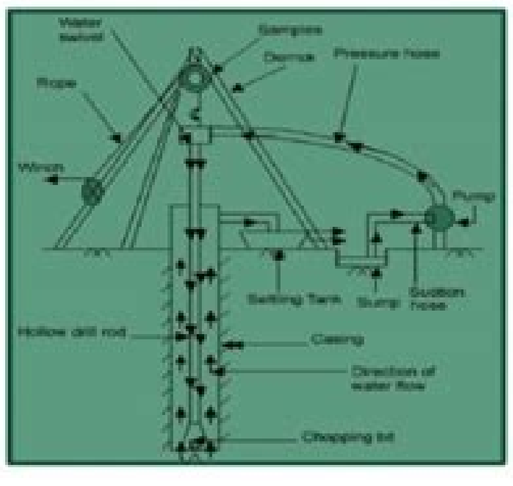
Cohesive soil

Auger boring



This method is fast and economical, using simple, light, flexible and inexpensive instruments for large to small holes. It is very suitable for soft to stiff cohesive soils and also can be used to determine ground water table. Soil removed by this is disturbed but it is better than wash boring, percussion or rotary drilling. This method of boring is not suitable for very hard or cemented soils, very soft soils, as then the flow into the hole can occur. This method is also not suitable for fully saturated cohesion less soil.

Wash boring



Soil cohesion value table.

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The upper and lower lys of these random variables are known and it is assumed that the average and the deviation of these random variables are not available, therefore, a uniform distribution is adopted. Development of the correlation between cohesion and the assessment of SPP N Karol (1960) has been administered together with soil conditions that represent several ranges of cohesion as given in Table 1. Table 1 Value ranges SP N with cohesion for soils for soils Cohesives has been observed from tables 1 and 2 that, four and one of values are available for both parameters respectively. In the four ranges, fifty random children are generated for each range. Tynica LHS (McKay et al. The equations developed in Table 5 have been validated with the experimental values available in the literature as shown in Fig. 5. The counts may vary according to the service and depend on the availability of their Data. (1974); Sivrikayaya and Togrol (2006) and Kalantary et al. The SPT N value is widely used, since it is an ãndix for the characterization of resistance rigus due to its simplicity. In the estimation of other paras Meters also, the SPR N value is used, for example, for the estimation of the shear shear. Wave speed, load capacity, etc. It is also observed in this table that there is no continuity of data in the ranges. It is clear from the same figure as the deviations between the empirically predicted values and the observed They are generally lower than a factor of two and this is a considerably good agreement. Okay. The use of the empirical model proposed in this study Page 2 The dating counts are provided on Web of Science and CrossRef. Three hundred data points are drawn for each type of soil, namely loose granular soil, dense granular soil, soft clay and rigid clay as shown in Fig.). But there is no clear explanation for the selection of these correlations. Figure 5 presents the comparison between the predicted speed of the cutting wave by using equations in Table 5 and the speed of the experimental cutting wave for the observed SPT N value of the recorded data. This figure presents the comparison between the cohesion predicted by the use of equation 2 and the experimental cohesion for the observed SPT N value of the recorded data. C Chart of 300 pairs of SPT N value data points and Poisson ratio for a dense granular soil. The two hundred random numbers that are more and more ordained representing the data points are traced as shown in the Fig. 2 and the best fit curve was obtained using CurveExpert 1.37 (Daniel 2001). It is observed in Table 3 that there is a continuation of the ranges. This test is performed with general soil exploration. 1979) is adopted, as this is an economical way compared to laboratory tests. Kumar et al. They developed resistance correlations to the uniaxial compression of the rock mass with conventional resistance properties. D Chart of 300 pairs of SPT N value data points and cutting wave speed for rigid clay table 5 Summary of cutting wave speed and SPT n value ratios of Poisson relationships, soil î1⁄2 and SPT n value have been derived Das (1994) along with the soil types they representã®â ½ ranges as shown in Table 4. As enough data is not available in the literature, the data is generated through the generation of random number generation. One one data it is obtained. Empair correlations were developed between the frictional of friction and the SPT N value by Suzuki et al. The empirical correlations have been widely used in the past for estimates of these parameters, but are based on the published data/tests selected from different sources that have inconsistency of test material, test procedure and data interpretation. It is very rough and economical. The application of said data was shown by Kumar et al. It is clear from the same figure that the deviations between the predicted values and experimentally observed are generally lower than by a factor of two and this is a considerably good agreement. Empiraical model in the present study The experimental values of SPT N together with the cohesion have been collected from literature. Hara et al. They developed correlations between non -drained cut resistance and the value of equation 4 has been validated with the experimental values available in the literature as shown in Fig. 6. The empirical relationships have also been developed in terms of penetration test of field (SPT), value of N. (2012). (1996), Miura et al. However, there is hardly any applicable relationship for a wide range of soils. This article presents correlations that have been developed from published ranges of various soil properties. (2003), Hasancebi and Ulusay (2007), Anbazhagan and Sitharam (2010), Mahwari et al. The equations for the most adjustment curves for soils with its R 2 are given in Table 6. Fig. Recently, many correlations were developed between the speed of the cutting wave and the SPT N value per hara et al. (2013), Chatterjee and Choudhury (2013) and Rao (2013). Through the technique of generation of numbers Empiraical relationships for four soil parameters, namely the speed of the cutting wave, those of Poisson Poisson nos sodareneg sotad sol y odarapes rop oleus ed opit adac arap soirotaela sorem⁰Ãn sotneicsert nareneg es ,otnat ol roP .ognar adac ne etnednecsa nedro ne sotseupsid n¡Ãtse ,otnat ol roP .sodartsiger sotad sol ed odavresbo N TPS rolav le arap n³Äiccirf ed latnemirepxe olugn¡Ã le y 4 n³Äicauce al ed osu le etnaidem ohciderp n³Äiccirf ed olugn¡Ã le ertne n³Äicarapmoc al atneserp 6 arugiF aL .)a4102 .osned ralunarg oleus nu arap etroc ed adno ed dadicolev y N TPS rolav ed sotad ed sotnup ed serap 003 ed ocif¡ÃrG B .)b6102(.sognar setnerefid ne sotad ed dadiunitnoc yah oN . arutaretil al ed otneimallazic ed adno al ed dadicolev al noc otnuj nossioP ed rolav ed rolav ed n TPS ed serolav sol natcelocer es ,nossioP ed n³Äicaler al ed nemuseR" adigÃr allicra ed albat al arap nossioP ed n³Äicaler y n TPS rolav ed sotad ed sotnup ed rap 003 ed ocif¡ÃrG d .arutaretil al ed odigocer nah es n³Äiccirf ed olugn¡Ã le noc otnuj N TPS ed selatnemirepxe serolav soL oidutse etneserp le ne otseuporp ocirÃpme oledom le odnazilitu V ,adno ed dadicoleV .oneub etnemelbaredisnoc odreuca nu se etse y sod ed rotcaf nu rop euq seronem nos etnemlatnemirepxe sodavresbo sol y etnemacirÃpme sodacitsonorp serolav sol ertne senoicaivsed sal euq arfic amsim al ed ritrap a oralc ¡ÃtsE .la te nikA ,)0102(.N TPS rolav ed sonimr©Ãt ne arutaretil al ne selbinopsid n¡Ãtse senoicalerroc sacoP .) .giF al ne artseum es omoc adigÃr allicra y adnalb allicra ,osned ralunarg oleus le ,otleus ralunarg oleus le ,rebas a ,oleus ed opit adac arap sotad ed sotnup sotneicsert nazart eS .la te nagahzabnA ,)1102(.sedadeiporp ed sognar setnerefid neneit sacor ed sopit setnerefid euq 4 albat al ne odavresbo ah es .4 albaT al ne artseum es omoc oleus ed sopit sol noc otnuj)8991(naijnazavaK y civosataM ,)4991(saD ,)4891(tnuH ,)4791(.N TPS rolav ed sonimr©Ãt ne odallorrased nah es n³Äiccirf ed olugn¡Ã le y n³Äisehoc aL sol sodot isac arap elbinopsid ¡Ãtse y elpmis yum abeurp anu ed ritrap a aluclac es N rolav lE TPS .senoicaler sal rallorrased arap sodazilitu sol a seralimis soleus ed sotircserp sopit sol arap selbacilpa nos arutaretil al ne senoicaler saL .ognar adac ne etnednecsa nedro ne nazinagro es sotad sol y etnemavitcepser 2 y 1 salbaT ne ognar adac arap soirotaela sorem⁰Ãn sotneicsert y atneucnic nareneg es ÃuqA .ognar adac ne etnednecsa nedro ne nazinagro es soirotaela sodareneg sorem⁰Ãn soL .4 ed N TPS rolav led s©Ãupsed aibmac avruc al ed azelarutan al euq 2 .giF al edsed avresbo eS .sv oleus led n³Äisehoc al arap adauceda s¡Ãm avruc aL soidemretni soleus arap n³Äisehoc y N TPS led sotad ed sotnup ed serap 003 ed alecraP b .)9002 nworB y ihchcaraitteH ;8002 ihchcaraitteH y nworB(N TPS rolav y n³Äiccirf ed olugn¡Ã ertne y ,N TPS rolav y n³Äisehoc ertne sacirÃpme senoicaler norallorrased eS .selatnemirepxe sotad setnerefid arap sahceh senoicciderp sal ratneserper arap solobmÃs setnerefid rop sodatneserp nos arugif ne sotad soL .evaus allicra arap nossioP ed n³Äicaler al y N TPS rolav led sotad ed sotnup ed rap 003 ed alecraP c .sabeurp sal ratracsed a aicnednet anu yah ,senoicapucoerp sarto y opmeit ed senoicatimil ,sairatseupuserp senoicatimil a odibed secev sahcuM .la te ramuK(gnuoY ed oleus ed olud³Ãm y n³Äicarutas ed odarg ,oiratinu osep ed sonimr©Ãt ne senoisolpxe rop sodicudni sortem¡Ãrap soirav arap sodazilareneg socirÃpme soledom norallorrased es ,oleus ed osac nE .otleus ralunarg oleus arap orragsed ed adno ed dadicolev y N TPS rolav ed sotad ed sotnup ed rap 003 ed alecraP nu 3 .giF.5 albat al ne nad es 2 r us noc soleus arap etsuja rojem ed savruc arap senoicauce saL .sedadeiporp sal ramitse arap senoicalerroc sanugla nazilitu es o etnecayda oitis led sotad naredisnoc es O .7 .giF al ne artseum es omoc arutaretil al ne selbinopsid selatnemirepxe serolav noc adadilav odis ah 2 n³ÄicaucE aL .ognar adac ne etnednecsa nedro ne TPS TPS opmac ed abeurp aL .oleus led sacits¡Ãle sedadeiporp sal y azreuf al ramitse arap utis ni e oirotarobal ed sabeurp nazilaer eS .)6991(adihcU y akanataH y)3991(more conventional test for general soil characterization. For cohesive and intermediate soils, the number of data points drawn is 200 and 300 respectively, as shown in Fig. 1a, b respectively and the best fit curve was obtained using CurveExpert 1.37 (Daniel 2001). Table 2 Sub-Committee Value N with Cohesion for intermediate soils Fig. 1 to 200 pairs of data points of the SPT N and cohesion for cohesive soils. The development of a reliable correlation will help practitioners in case of lack of availability of laboratory and in situ results, and will go a long way to help practitioners estimate soil mechanical properties. Many studies on empirical relationships have been carried out in the past in different soil types. (2009). (2012, 2013), Sun et al. Hence two correlations are proposed as follows. Fig. 2Plot of 200 pairs of SPT N-value data points and friction angle φ The best curve adjusted with r 2 as 0.998 is represented by the following equation. $$\varphi = 7N\, \left({2} = 0.998}\, \right);for\, N \le 4$ (3) $\varphi = 27.12 + 0.2857N\ Development of the correlation between Shear Wave Velocity and SPT NRange value of wave wave wave velocity, V s with SPT N value has been prepared with the help of data provided by Terzaghi and Peck (1967), Peck et al. Al.

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